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CONDITION SURVEY, HUNTER ARMY AIRFIELD SAVANNAH, GEORGIA

R. D. Jackson, et al

Army Engineer Waterways Experiment Station Vicksburg, Mississippi

March 1972

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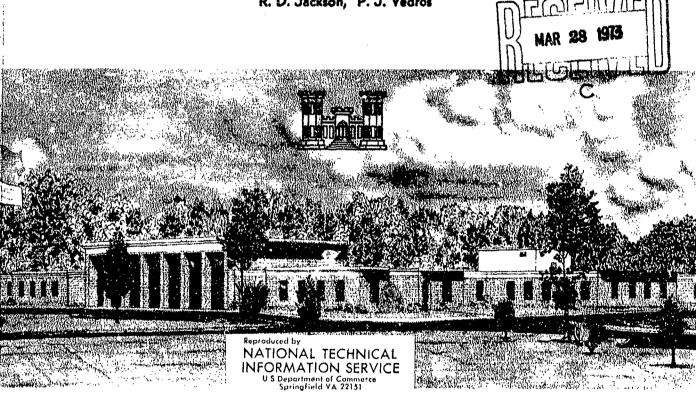
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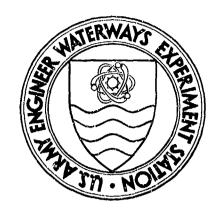


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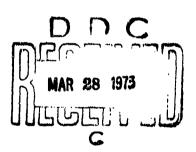
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#### Foreword

Authority for performance of condition surveys at selected air-fields is contained in Long Range Program, O&M,A FY 1971, Project Q6-1: "Engineering Criteria for Design and Construction - WES," dated May 1970.

The facilities at Hunter Army Airfield were inspected in March 1971 by Messrs. R. D. Jackson and S. J. Alford of the Flexible Pavement Branch, U. S. Army Engineer Waterways Experiment Station (WES). This report was prepared by Messrs. Jackson and P. J. Vedros under the general supervision of Messrs. J. P. Sale, R. G. Ahlvin, and R. L. Hutchinson of the Soils Division, WES.

COL Ernest D. Peixotto, CE, was Director of the WES during the conduct of the study and preparation of the report. Mr. F. R. Brown was Technical Director.

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### Conversion Factors, British to Metric Units of Measurements

British units of measurement used in this report can be converted to metric units as follows:

Multiply	Ву	To Obtain
inches	2.54	centimeters
feet	0.3048	meters
square inches	6.4516	square centimeters
pounds	0.45359237	kilograms
pounds per square inch	0.6894757	newtons per square centimeter

# CONDITION SURVEY, HUNTER ARMY AIRFIELD SAVANNAH, GEORGIA

#### Purpose

1. The purpose of this report is to present the results of an inspection performed at Hunter Army Airfield (HAAF) in March 1971. The inspection was limited to visual observations, and no tests were conducted on any of the pavement facilities. A layout of the airfield is shown in plate 1.

#### Pertinent Background Data

### General description of airfield

- 2. HAAF, formerly Hunter Air Force Base, is located in the south-west corner of Savannah, Georgia.
- 3. The airfield is located physiographically in the Sea Island section of the Coastal Plain province in an area of gently rolling topography. The soil in the area is generally a poorly graded sand, with scattered deposits of fine sand, silt, and lean clay. However, at lower depths occasional pockets of fat clays are found.
- 4. In March 1971, the airfield facilities consisted of an east-west runway 11,375 ft\* long and 200 ft wide, connecting taxiways, parking aprons, two warm-up aprons, former alert aprons and taxiway, and a compass swing base (see plate 1). The taxiways and aprons are of various lengths

<sup>\*</sup> A table of factors for converting British units of measurements to metric units is presented on page vii.

and widths. Huey-type helicopters were utilizing the large parking apron and the former alert facilities for parking.

#### Previous reports

- 5. Previous reports covering the airfield facilities are listed telow and pertinent data were extracted from them for use in this condition survey report.
  - a. Condition survey reports:
    - (1) U. S. Army Engineer Waterways Experiment Station, CE, "Condition Survey, Hunter Army Airfield, Savannah, Georgia," Miscellaneous Paper S-69-37, August 1969, Vicksburg, Miss.
    - (2) U. S. Army Rigid Pavement Laboratory, Ohio River Division, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," July 1959, Mariemont, Ohio.
    - (3) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Georgia," October 1956, Mariemont, Ohio.
    - (4) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," July 1953, Mariemont, Ohio.
    - (5) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," April 1951, Mariement, Ohio.
  - b. Evaluation reports:
    - (1) U. S. Army Engineer Waterways Experiment Station, CE, "Airfield Pavement Evaluation, Hunter Air Force Base, Savannah, Georgia," Miscellaneous Paper No. 4-379, February 1960, Vicksburg, Miss.
    - (2) U. S. Army Engineer Waterways Experiment Station, CE, "Airfield Pavement Evaluation, Hunter Air Force Base, Savannah, Georgia," April 1958, Vicksburg, Miss.
    - (3) U. S. Army, Savannah District, CE, "Airfield Evaluation, Final Report," April 1945, Savannah, Georgia.

#### History of Airfield Pavements

#### Construction history

6. The majority of pavement facilities were constructed during the period of years 1941-1959. A summary of the construction history (from the evaluation report, reference 1, paragraph 5b) is shown in table 1. The pavements constructed after 1955 were designed to support a landing gear load of 100,000 lb carried on dual wheels spaced 37.5 in. c-c, each wheel having a tire contact area of 267 sq in. Typical sections of the primary runway and taxiways are shown in plates 2 and 3, respectively. Pavement thickness and other details for all pavement features are shown in the summary of physical property data shown in table 2.

#### Traffic history

7. During 1967, HAAF was converted from an Air Force to an Army installation. Prior to the change, the pavements were utilized by heavy bomber and cargo-type aircraft. The Army is using the facilities for rotary-wing aircraft used for pilot training. Considerable traffic is recorded for Huey-type helicopters; however, these aircraft have little adverse effect on the pavements, which were designed for heavy loads. The runway and taxiway pavements are generally used for small Army aircraft; however, occasional use is made of the system by transient Air Force heavy-type aircraft.

#### Condition of Pavement Surfaces

8. A visual inspection of the pavements in March 1971 indicated the airfield pavement to be generally in good condition.

#### Flexible pavement surfaces

9. The surface of the east-west runway was excellent (photo-graphs 1 and 2). The surfaces of the taxiways, aprons, and holding areas

at taxiways 1 and 5 presented a good appearance. The areas of the taxiways and aprons that were treated with a bituminous coating material in 1967 and 1968 (condition survey report, reference 1, paragraph 5a) showed numerous open cracks and scaling of the coating in some areas (photographs 3-6). A white deposit observed along the cracks in the holding pad at taxiway 5 is shown in photograph 7. The deposit was a fine-grained powder that apparently came from the limerock base course. Asphalt shoulder pavements of the former alert facilities are being utilized by Cobra-type helicopters as parking areas. Considerable deterioration of the pavement has occurred because of fuel spillage (photographs 8 and 9). The shoulder pavements of taxiway 6 had many cracks with vegetation growing in the cracks.

#### Rigid pavement facilities

vealed that the majority of these pavements were in excellent condition. The pavement of the east apron and the small slabs of the holding pads (former alert parking aprons) were rated good. The defects in the former alert facilities areas probably were developed while they were being utilized by heavy Air Force aircraft. It seems unlikely that helicopters could have caused and defects. The slabs in apron B were in fair condition. Typical exects in this area are shown in photographs 10-14. Photograph 19 years was facel stains at a Huey parking spot on the west apron.

#### Airfield Maintenance

#### Recent maintenance

11. During 1970 the asphalt portion of the east-west runway was heater planed to remove the oxidized pavement surface. It was then rolled and a slurry seal was applied.

#### Planned maintenance

12. Maintenance scheduled for rigid pavement facilities during 1971 included replacing shattered slabs, cleaning and repairing cracks in broken slabs, and applying joint sealer to a large portion of the east apron and apron B. After such maintenance, these areas should be in good condition. Maintenance planned for the flexible pavements during 1971 was slurry-sealing of taxiways 1-3 and the holding area adjacent to taxiway 1. Also, the shoulder pavements of taxiway 6 were to be removed and replaced with hot-mix asphaltic concrete. An everlay of the flexible pavement of the overrun area at both ends of the eastwest runway is planned for FY 1972.

#### Evaluation

13. The evaluation of the load-carrying capacity of the HAAF paverents was based on criteria contained in TM 5-827-2 and TM 5-827-3, "Flexible Airfield Pavement Evaluation," and "Rigid Airfield Pavement Evaluation," respectively, and on the strength values assigned for the 1960 evaluation. Evaluations are shown in table 4 for four life categories of airfield pavements and various types of landing gear wheel assemblies. An aircraft identification index is presented in table 5, which lists the various types of aircraft according to landing gear configurations.

Table 1
Construction History

	<del> </del>	<del></del>	Paveme	nt		
Facility	Length ft	Width ft	Thickness in.	mune.	Construct Period	tion Agency
	1 1/		111.	Туре	101100	ngency
H-W runway	10.000	200	4	4.0	1001 1050	QE.
Sta 0+00-105+00 Sta 95+00-105+00	10,500	200	4	AC	1951-1952	CE
(strengthened)	1,000	200	8	AC	1955-1956	CE
Sta 1:5+00+113+75 Sta 0+60-3+00	875 300	200 200	15 19 <b>-</b> 22	PCC PCC	1955-1956 1957	CE IE
Sta 3+00-105+00	300	200	19-22	100	エンフリ	711
(strengthened)	10,200	500	1.	AC	1959	IE
Alert aprons and twy			50	PCC	1959	CE
Texiway 6	1.,300 <u>+</u>	<b>7</b> 5	18	PCC	1957	CE
Taxiway 5						
Original	5,400 <u>+</u>	100	14	AC	1951-1952	CE
Sta 62+50-83+00 (strengthened)	2,050	80	1-1/2	AC	1959	IE
Taxiway 1	1,670+	<b>7</b> 5	4	AC	1951-1952	CE
Taxiway 1	670+	75	4	AC	1951-1952	CE
·	010±	17	4	AC	17/1-17/2	CE
Taxiway 3	(nn.	F3 (**	ŽĻ.	4.0	1051 105 <b>0</b>	Ord
Southwest end Northeast end	(30 <u>+</u> 2,200∓	75 150	6	AC PCC	1951-1952 1941	CE CE
Strengthened	2,200+	150	14	AC	1952-1953	CE
Taxiway 2						
Southeast end	970+	<b>7</b> 5	14	ΛC	1951-1952	CE
Northwest end	900+	150	6 4	PCC	1.941	CE
Strengthened	900+	150	4	AC	1952-1953	CE
E-W taxiway			,		ret.	<b>a</b> 12
Original Strengthened	5,300 5,300	150 150	6 4	PCC AC	1941 1952-1953	CE CE
Hangar aprons	7,500	-,-	13	PCC	1953-1954	CE
Compass swing base			15	PCC	1953-1954	CE
West apron			15	PCC	1953-1954	CE
Apron B			6	PCC	1942	CE
East apron					·	
Original			6	PCC	T3 <sub>45</sub>	CE
Strengthened			11	PCC	1955-1956	CE
North apron			15	PCC	1955-1956	CE
South apron			1.5	PCC	1953-1954	CE
Apron A			15	PCC	1953	CE

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FACILITY				OVEPLAY PAVEMENT		PAVEMENT			BASE		SUBGRACE		
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FACILITY				OVERLAY PAVEMENT			PAVEMENT			BASE		SUBGRADE	$\vdash$	
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 $\label{eq:Table 4}$  SUMMARY OF PAVEMENT EVALUATION

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tationd				1	7	Asphaltic concrete		c	Portlanl- cement concrete	650				Stnd	350	Emergency Mini.um Full Capacity	155,000+ 155,000+ 155,000+ 115,000	65,0
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Table 4
SUMMARY OF PAVENER'S EVALUATION

					CARACIA	V 19 1 8 0F 6	OSS PLANE LO	AD FOR INDICA	TED LANDING	GEAR			
SUBGRADE				LOAD CAR	TYPES	AND CONFIGU	RATIONS AND L	IFE CATEGORI	ES				TRAFFIC
		CATEGORY OF					YCLE ARRANGE					PICYCLE 1 MIN 1 MIN 5PCG 1'-62-17	AREA
LASSIFICATION	CBH	PAVEMENT LIFE AND OPERATIONAL USE	SINGLE IP: PSI SING PRESSURF	SINGLE 100 SQ IN CONTACT SHEA	SINGLE 24) 10 IN CONTACT ANEA	TAJE C-C 320 5Q IN CONTACT AMEN KACH TINE	SINGLE TANCEM SO SPACING SOO SO IN CONTACT SHEA	TR 11 C:C 247 5Q IN CONTACT ANEA EACH TINE	THE C C 400 50 IN CONTACT ANTA EACH TIME	37 - 48 208 BQ IN CONTACT AREA ERCH TIME	C-4A GEAR COMPIGURATION	\$ 9 Cm 4 mb. 8   6 Cm 1 ec. 4	
		Fmergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+ 380,000+	770,000+ 770,000+	600,000+ 600,000+	
		Minimum Full	155,000+ 155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 330,000+	230,000+	380,000+ 380,000+	770,000+ 770,000+	600,000+	Α .
t	300	Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Amergency Minimum	155,000+ 155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 330,000+ 330,000+	230,000+ 230,000+	380,000+ 380,000+	770,000+ 770,000+	600,000+ 500,000	
,	25	Full Capacity	155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+ 185,000	200,000+	250,000+	230,000+	380,000	770,000+	390,000	В
		Pargency	155,000+	65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 330,000+	230,000+ 230,000+	380,000+ 380,000+	770,000+	600,000+	
		Minimum Full	155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 250,000	230,000+ 230,000+	380,000+	770,000+	580,000 440,000	С
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		Amergency Minimum Full	155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+ 220,000+	200,000+	330,000+ 330,000+	230,000+	380,000+	770,000+	600,000+	_
t	۲۰,	Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000	<u> </u>
		Emergency Minimum	155,000+ 155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+ 220,000+	200,000+	330,000+ 330,000+	230,000+	380,000+ 380,000+	770,000+ 770,000+	500,000+	
_		Full Capacity	155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	295,000+ 230,000+	230,000+	380,000+ 380,000+	770,000+ 770,000+	430,000 360,000	A
4	300		155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Amergency Minimum Full	155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 330,000+	230,000+ 230,000+	380,000+	770,000+	600,000+ 560,000	
1	300	Capacity	155,000	65,000+	95,000+	220,000+	200,000+	310,000	230,000+	380,000+	770,000+	470,000	^
		Emeryency Minimum	155,000+ 155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	336,000+ 320,000	230,000+	380,000+ 380,000+	770,000+	€00,000÷	
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		Hmergency Minimum	155,000+ 155,000+	65,000+ 65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
4	25	Full Capacity	155,000+ 155,000+	65,000+	95,000+ 95,000+	220,000+ 155,000	200,000+	330,000+	230,000+	380,000+ 380,000+	770,000+ 770,000+	500,000	С
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et.	9.0	Pull Capacity	155,000+ 115,000	65,000+ 65,000+	95,000+ 95,000+	220,000+ 175,000+	200,000+	180,000	230,000+	360,000 290,000	€00,000	245,000	A .
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)		Minimum Full	155,000+ 155,000+	65,000+	95,000+ 95,000+	220,000+	200,000+	330,000+ 330,000+ 290,000	230,000+	380,000+ 380,000+	770,000+	410,000	
k!		Capacity	115,000	65,000+	95,000+	175,000	200,000+	230,000	230,000+	380,000+	770,000+	370,000	c
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Table 4 (Concluded)

FACILITY		T	CVERLAY PAVEMENT			Г	PAVEMENT			BASE	SUBGRADE		LOAD CARRYING					
IDENTIFICATION	. Enarm	A.tı Ţ	, FE	,	DESCRIPTION	FLEX. SYN PSI	THICK.	DESCRIPTION	FLEX STR	THICK.	DESCRIPTION	CDR	CLASSIFICATION	Can	CATEGORY OF PAVEMENT LIFE AND OPERATIONAL	\$1746Lb. 100 PSi	\$1M\$LE	— Т
Past Apron	Select	ed fi	gures	-	Portland-			Portland- cement concrete							Energency Minimum Full	155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+	co
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(		Sela	c ed	fgure	for evaluation		14	Asphaltic concrete		6	Waterbound macadam Subbase	80 40	Sand	25	Finergency Minimum Pull Capacity	155,000+ 155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+ 65,000+	
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	Select for	pd fi	ares	4	Asphaltic concrete		6	ET 10.7 Fortland- cement concrete	650				Sand	350	Fmergency Minimum Full Capacity	155,000+ 155,000+ 145,000	65,000+ 65,000+ 65,000+ 65,000+	
pron B		Selec	64	vures	for evaluation		6	Portland- coment concrete	650				Sand	350	Fmergency Minimum Full Capacity	90,000 150,000 110,000 85,000 55,000	65,000+ 65,000+ 60,000 40,000	
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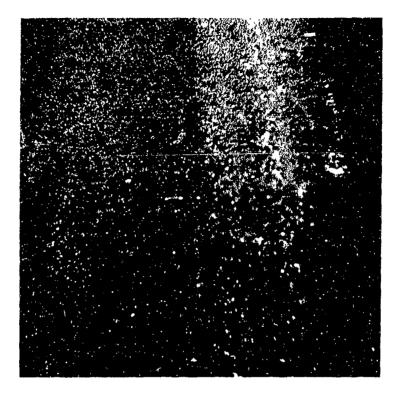
Table 1 (Concluded)

SUBGRADE			LOAD CARRYING CAPACITY IN LB OF GROSS PLANE LOAD FOR INDICATED LANDING GEAR TYPES AND CONFIGURATIONS AND LIFE CATEGORIES											
	Γi	LATEGUMY OF PAYENCHT LIFE AND OPERATIONS UNE	TRICYCLE ARMAYOSMENT									a Che	TRAFFIC ARFA	
. ASSIBICATION	Can		S NOLE 1 to PBI TING PRESSURE	9146.6 130 BQ IN CUNTACT AREA	614618 341 89 14 CONTACT AND 8	TO 48 E C	SINGLE PANDEM AN SPACING ANC SEIN CONTACT ANDA	TO 17 C C 307 SQ IN CD41 GCT ANY G SACH TIRE	THE C C 610 BQ IN CONTACT AME B EACH TIME	TRIN TENDEM SE + 46 JUE BO IN CONTRET AMER EACH TIME	C BA GT AN CONFIDURATION	TOM TOM BHC 6 7" 62 12 141 55 IN COL164 TAND 6 4 50 TIME		
'and	350	Emergency Minimum Sull Cupacity	155,000+ 155,000+ 155,000+ 150,000	65,000+ 65,000+ 65,000+ 65,000+	95,000+ 95,000+ 95,000+ 95,000+	220,000+ 220,000+ 220,000+	200,000+ 200,000+ 200,000+	330,000+ 330,000+ 330,000+ 295,000	230,000+ 230,000+ 230,000+	380,000+ 380,000+ 380,000+ 380,000+	770,000+ 770,000+ 770,000+ 770,000+	600,000+ 600,000+ 600,000+ 1170,000	c	
Sund	225	Mergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+	95,000+ 95,000+ 95,000+ 95,000+	220,000+ 220,000+ 220,000+	200,000+ 200,000+ 200,000+	330,000+ 330,000+ 330,000+ 330,000+	230,000+ 230,000+ 230,000+ 230,000+	380,000+ 380,000+ 380,000+ 380,000+	770,000+ 770,000+ 770,000+ 770,000+	600,000+ 600,000+ 600,000+ 560,000	В	
und	25	Smergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+ 65,000+	95,000+ 95,000+ 95,000+ 95,000+	220,000+ 220,000+ 220,000+ 155,000	200,000+ 200,000+ 200,000+ 200,000+	330,000+ 330,000+ 310,000 220,000	230,000+ 230,000+ 230,000+ 230,000+	380,000 380,000+ 380,000+ 380,000+	770,000+ 770,000+ 770,000+ 770,000+	600,000+ 600,000+ 500,000 410,000	С	
and	හ	Prengency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 120,000	65,000+ 65,000+ 65,000+ 65,000+	95,000+ 95,000+ 95,000+ 85,000	220,000+ 220,000+ 150,000 120,000	200,000+ 200,000+ 200,000+ 200,000+	330,000+ 310,000 250,000 180,000	230,000+ 230,000+ 230,000+ 230,000+	380,000+ 380,000+ 330,000 270,000	770,000+ 770,000+ 770,000+ 770,000+	600,000+ (a.) (a.)	c	
erd	350	Spersency Minimum Pull Capacity	155,000 155,000 145,000 90,000	65,000+ 65,000+ 65,000+ 65,000+	95,000+ 95,000+ 95,000+ 95,000	220,000+ 220,000+ 215,000 140,000	200,000+ 200,000+ 200,000+ 200,000+	330,000+ 330,000+ 235,000 190,000	230,000+ 230,000+ 230,000+ 230,000+	380,000+ 380,000+ 380,000+ 380,000+	770,000+ 770,000+ 770,000+ 770,000+	600,000+ 500,000 340,000 305,000	·	
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Table 5

Aircraft Identification Index
(For Gear Configurations Shown in Columns 1-10, Table 4)

1	2	_3	4	5	6	_7_	- 8	9	10
B-26-B	B-45-C	F-111	C-119	C130		C-124	C-133	C5A	B-52
B-45-A	F-84-T		C-54-G		B-50		C-135		B-52-A
B-57-B	F-84-G		C-118		KC-97		KC-135		
B-66-C	F-86-D		C-131		C-74		C-141		
C-45-F	F-86-F				C-121		KC-137		
C-45-G	F-86-H								
C-46-F	F-89 Series								
C-82	F-100 -A								
C-123-B	F-101-A								
F-86-A	F-102								
F-86-E	C-47								
F-94-B	B-57								



Photograph 1. Surface typical of asphalt portion of east-west runway



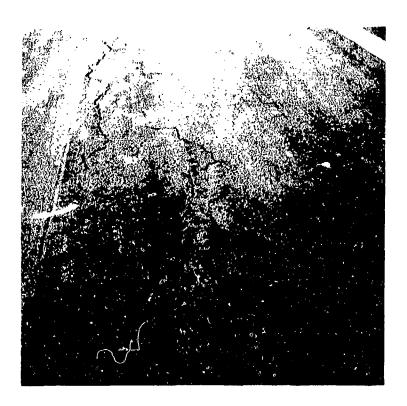
Photograph 2. Closeup view of slurry seal on east-west runway



Photograph 3. Open cracks near east end of the asphalt apron



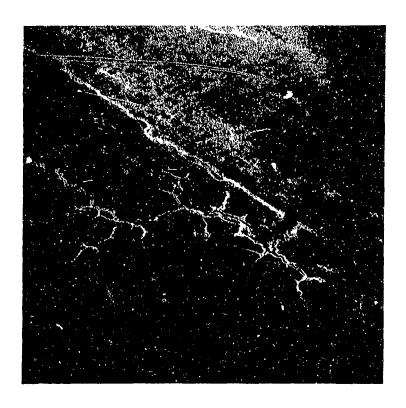
Photograph 4. Longitudinal cracks in taxiway 4



Photograph 5. Random cracking, taxiway 5



Photograph 6. Scaling of bituminous seal, east apron area



Photograph 7. White powdery substance along cracks in pavement of holding area at taxiway 5



Photograph 8. Effects of fuel spillage on shoulder pavements of former alert area



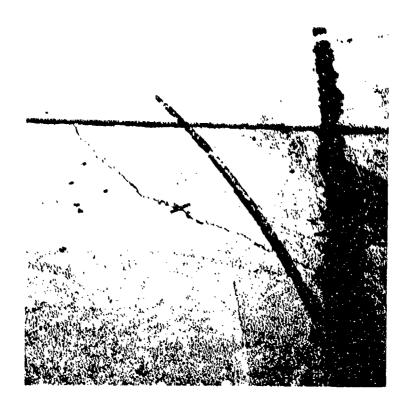
Photograph 9. Ruts in fuel spillage area on shoulder pavements of former alert area



Photograph 10. Shattered slab in apron B



Photograph 11. Spall along transverse crack, apron B



Photograph 12. Corner break of slab, apron B



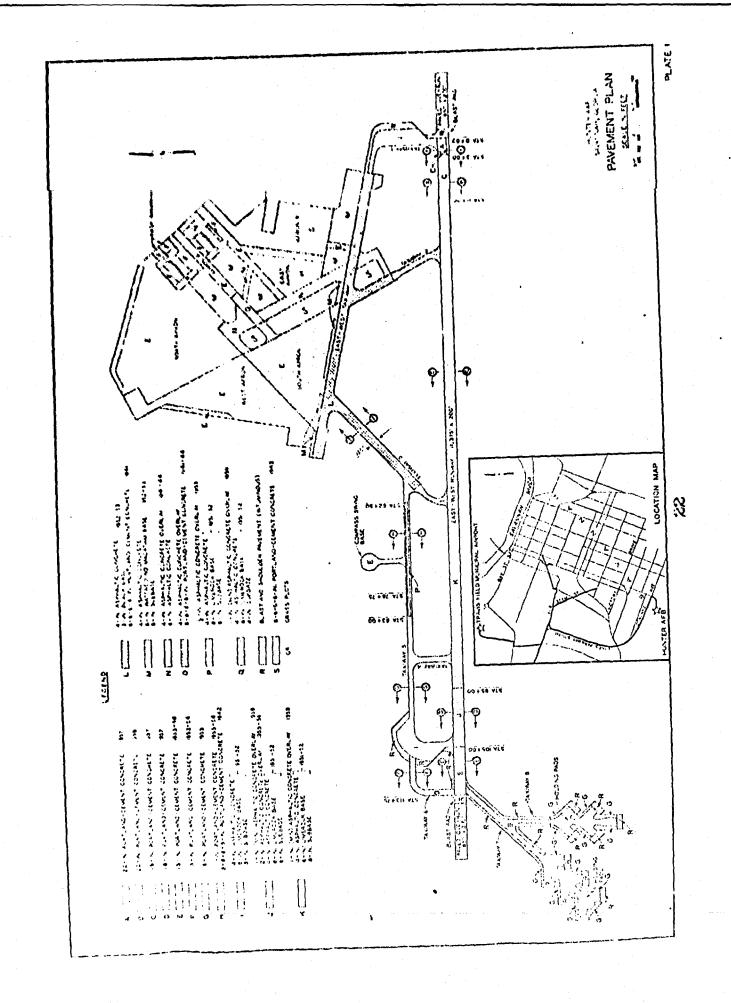
Photograph 13. Typical popout in rigid pavement, apron B

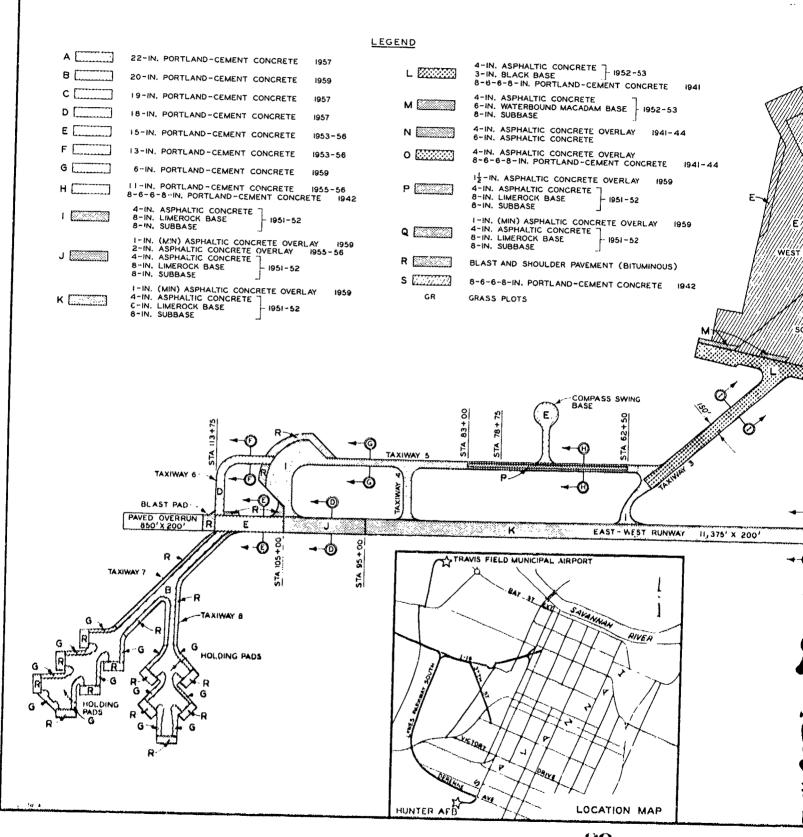


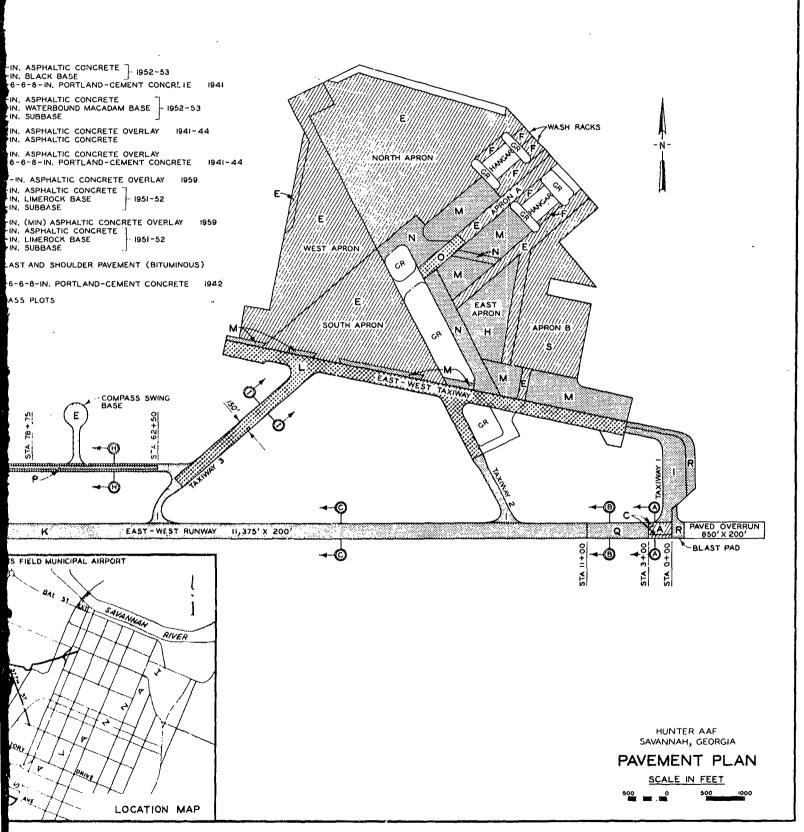
Photograph 14. Spalling along longitudinal joint, apron B



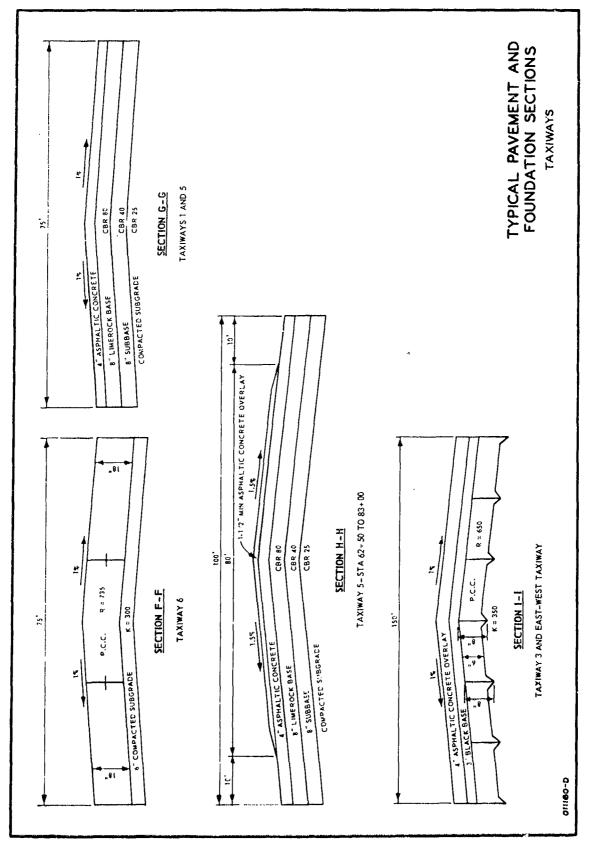
Photograph 15. Fuel stains at Huey parking spot, west apron







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PLATE 3